



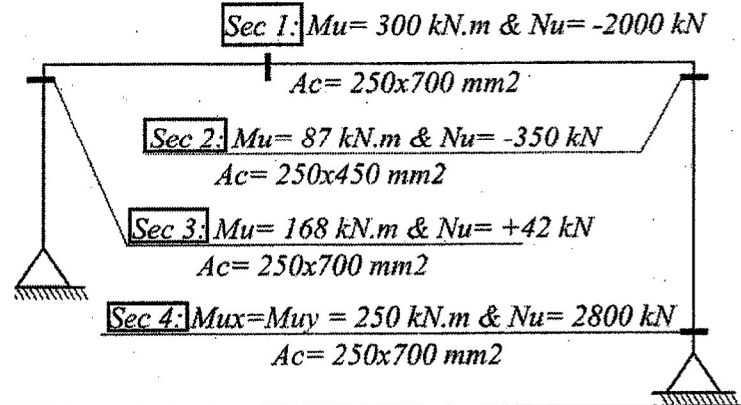
- Answer all the following question.
- Any missing data may be reasonably assumed.
- This course satisfy ILOS of A4, A5 and A6- B4, B5 and B11- C3, C4 and C6- D1, D2, D6 and D7

يسمح باستخدام جداول و مساعدات التصميم الخرسانية

Question No. 1 (20%):-

- a) a) Mention the zones of interaction diagram showing only define and conditions for each one.
- b) Design the critical sections of frame shown in Fig. 1. $F_{cu}=35 \text{ N/mm}^2$, $f_y=360 \text{ N/mm}^2$

Fig. 1



Question No. 2 (20%):-

- a) Define with sketch if any:-

- The reinforced concrete walls, its types, reinforcement recommendations according to ECP. Mention with sketch how to reinforce the opening existing in R.C walls.
- Resisting deflection for flexural elements

باختصار تكلم عن استلام: الأعمدة (تجاره و حداده) - الكمرات (حداده) - البلاطات (حداده) - الحوائط المفترقه (حداده)

- Details of R.F.T for columns resisting seismic loads.

- b) The given longitudinal sections are for a two beams with given cross sections (A_c) and steel reinforcement (A_s) shown in Fig.2. It is required to draw the reinforcement details presenting longitudinal section and the determined cross section only.

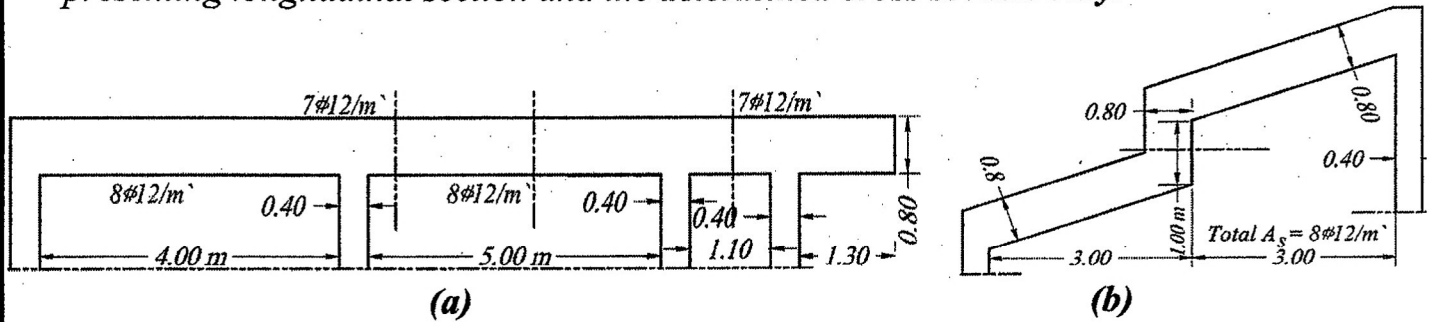


Fig. 2

Question No. 3 (65%):-

The given structural shown in Fig. 3 is for a building designed based on the following data:

Slab thickness (t_s) = 120 mm - Floor covering (F.C) = 1.50 kN/m² - Live Load (L.L) = 2.00 kN/m² - Wall density (γ_w) = 12 kN/m³ - Wall thickness = 250 mm - Number of floors = 10 floors - $f_{cu}=35 \text{ N/mm}^2$ & $f_y = 360 \text{ N/mm}^2$ - All beams have $A_c \text{ beam}=250 \times 700 \text{ mm}^2$ - the building is braced in both directions.

It is required to execute the follows:-

- a- It is required to check both deflection and cracking width for slab S.
- b- Provide a full design followed by reinforcement details with scale of 1:50 FOR ONLY strip I-I.
- c- Sketch with a suitable scale the total RFT of slabs for the whole plane.
- d- Estimate the load carried over column C1, followed by a design as a tied rectangular column. Then, calculate the new ultimate load (P_u as a composite column) if an IPE 330 with an area of 6260 mm^2 was added.
- e- With the same estimated load, Re-design column C1 as a spiral one.
- f- IF you know that the building is braced in both directions, design Column C2 (shown in plan and Fig. 4) with a predicted dimensions about $(300 \times 600) \text{ mm}^2$ to resist an ultimate load of $P_u = 3000 \text{ kN}$.
- g- Design the Tie member shown in Fig. 5.

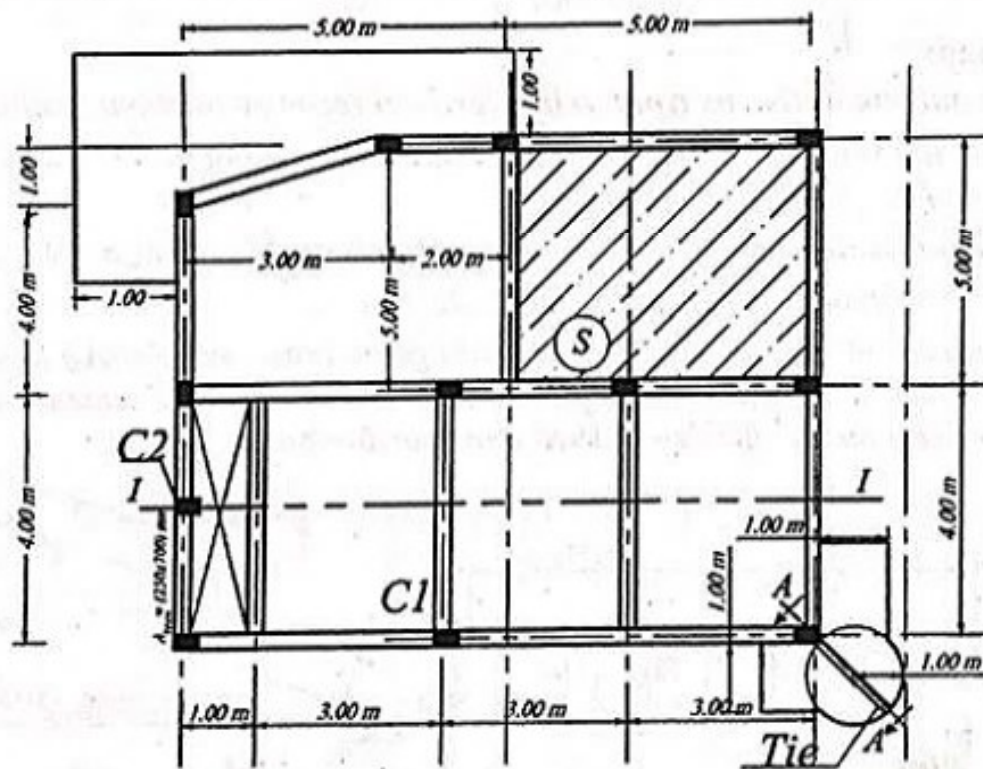


Fig. 3; PLAN

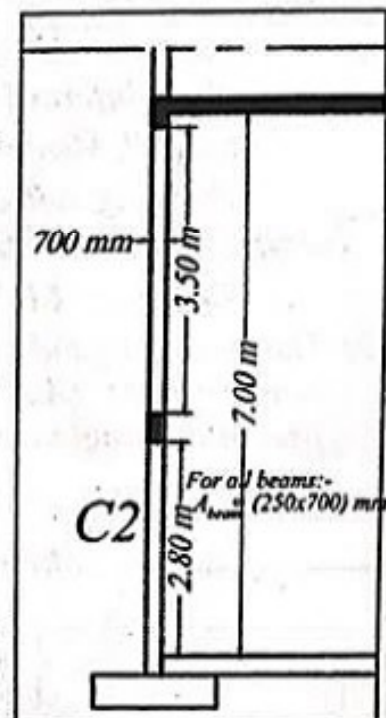


Fig. 4; Sec I-I

Fig. 5; Tie section

