



Notes: - Assume any missing data reasonably. - Only steel tables and formula sheets are allowed.
- Unless otherwise noted, steel used is St. 52

Question 1

1- The outline of an industrial halls shown in **Figure (1)**. Columns are not allowed inside the hall, it is required to choose suitable systems for covering this area under the following conditions:

- Main entrance is 8.0 m width at side B-C.
- Natural light must be well distributed on the hall.

It is required to draw with a suitable scale elevation, side view, end gable and plan showing the different arrangement of bracing system.

2- **Determine** the maximum ultimate load (w) that can be applied on the simple beam where its section is BF1B 400 for the both given cases, to satisfy **flexure strength and deflection** requirement;

1- **Case 1**; the top flange is continuously laterally supported by RC slab.

2- **Case 2**; the top flange is only laterally supported at ends.

3- **Design** a crane track girder with 6.0 m span using HEA section assuming the maximum reactions of the crane are two loads 15.0 tons each and spaced 1.5 m (Consider the dynamic effect (I) = 25% and the lateral shock = 10%).

Question 2

1- **Design** a column using four angle –open section from two sides- to carry a total load of 120 t, case I. The column is hinged at both ends with buckling length $L_{bx} = L_{by} = 7.0$ m, St 37, $\phi = 16$ mm with grade 4.6 [**Hint**: use double lacing bars]

2- **Discuss** with neat sketch the **buckling** length factor for members with well defined end conditions, **mention** example on each case

3- **Calculate** the buckling lengths for column shown in **Figure (2)**,. Assume there is vertical bracing outside plane connecting columns at floor levels only

Question 3

1- Design the hinged base connection ($N = 40$ t & $H = 5.0$ t) where the cross section of the column is two BF1B NO 300.

2- **Draw** the flow force diagram for the fixed base at joint (A) of **Figure (3)**.

3- **Design and draw** the rigid connection at joint (B) of the frame shown in **Figure (3)** if $N = -15.0$ ton, $Q = 10.0$ ton and $M = 25.0$ m.t., using [M 30 grade 8.8] pretensioned bolts. The rafter is HEA 500

4- A composite beams cover an area of 7.0×15.0 m² each beam is simply supported with span 7.0 m and spacing between beams is 1.5 m. The slab thickness 12.0 cm with hunch 7.0 cm. The floor cover = 150 kg/m², Live Load = 400 kg/cm². The interface between the concrete slab and the steel beam is assumed to be a full connection type. It is required to **design the composite beam to satisfy the flexure and deflection criteria**. Take St 37 and $F_{cu} = 300$ kg/cm² ($f_c = 95$ kg/cm²) [**Hint**: use minimum steel height]

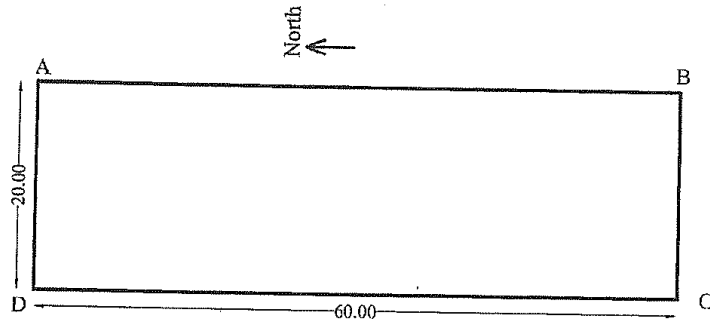


Figure 1

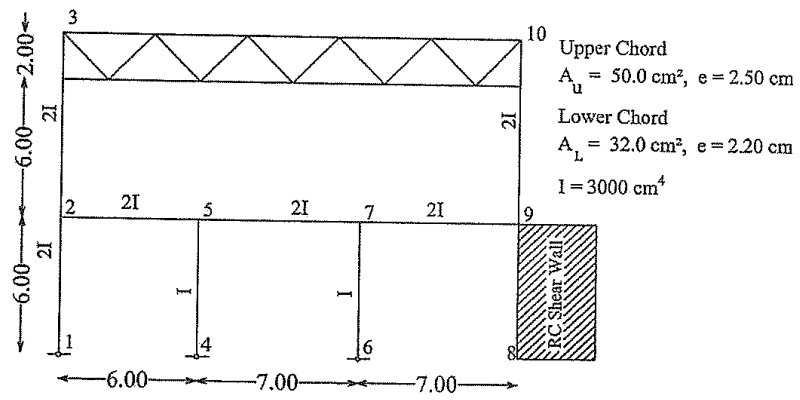


Figure 2

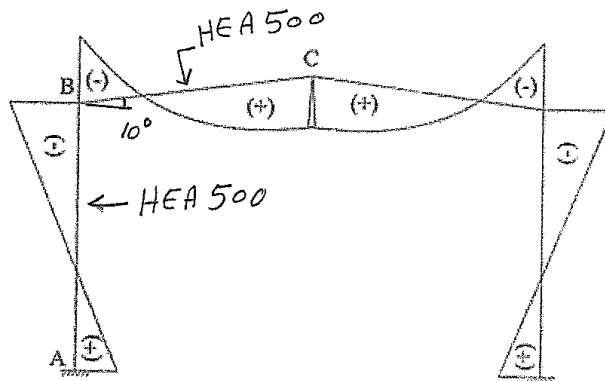


Figure 3

Best Wishes
Dr. Ali M. Basha